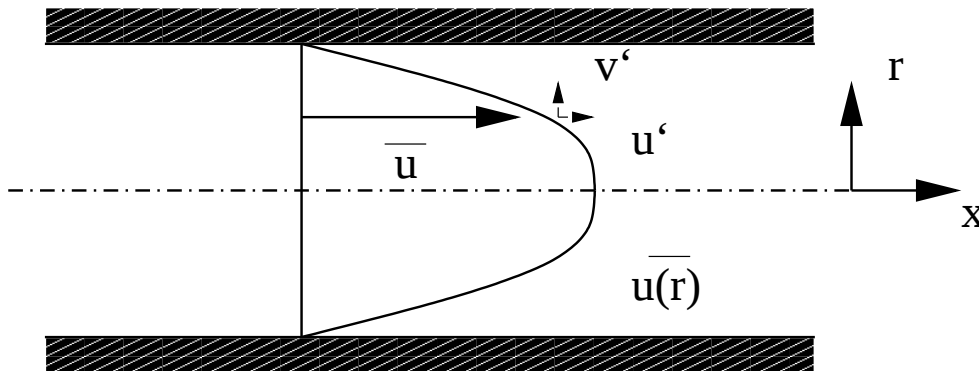


Turbulent flows

Reynolds averaging: splitting of the turbulent velocity $\vec{v}$ in an average value $\overline{\vec{v}}$ and a fluctuation $\vec{v}'$

$$\begin{array}{ccccc} \vec{v} & = & \overline{\vec{v}} & + & \vec{v}' \\ \uparrow & & \uparrow & & \uparrow \\ \text{total vector} & & \text{time average} & & \text{fluctuation} \end{array}$$

Example: Pipe



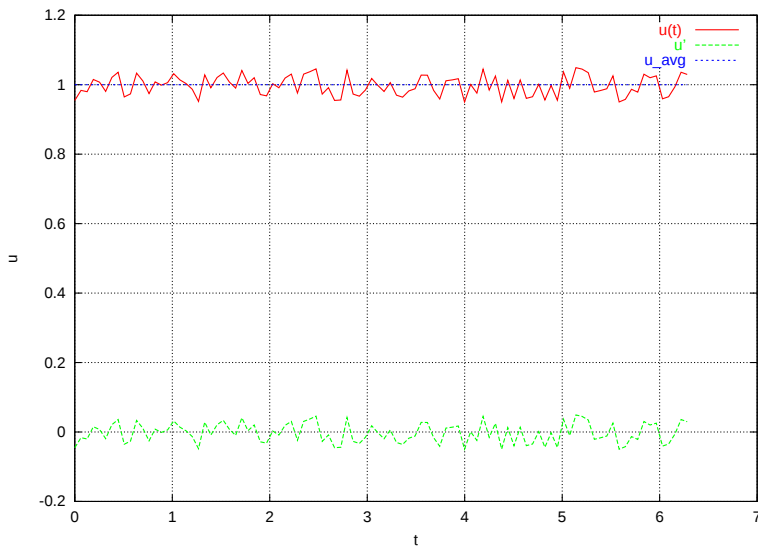
fully turbulent
symmetric flow

$$\begin{aligned}u(r, \phi, x, t) &= \bar{u}(r) + u'(r, \phi, x, t) \\v(r, \phi, x, t) &= v'(r, \phi, x, t)\end{aligned}$$

definition:

$$\bar{u} = \frac{1}{T} \int_T u(x, y, z, t) dt$$

$$\rightarrow \bar{u} = \bar{u}(x, y, z) \neq f(t) \quad u' = u - \bar{u}$$



$$\overline{f'} = 0 \quad \text{average of the fluctuation}$$

$$\overline{\overline{f}} = \underbrace{\overline{f}}_{\text{konst} \neq f(t)} \quad \text{average of the average}$$

$$\overline{f + g} = \frac{1}{T} \int_T (f + g) dt = \frac{1}{T} \int_T f dt + \frac{1}{T} \int_T g dt = \overline{f} + \overline{g}$$

$$\overline{f \overline{g}} = \overline{f} \overline{g} : \quad \overline{g} \neq \overline{g}(t) \rightarrow \frac{1}{T} \int_T f \overline{g} dt = \frac{1}{T} \overline{g} \int_T f dt = \overline{f} \overline{g}$$

$$\overline{\frac{\partial f}{\partial x}} = \frac{\partial \overline{f}}{\partial x} \quad \text{average of the derivative}$$

Computational rules

$$\overline{f g} = \frac{1}{T} \int_T f g \, dt = \frac{1}{T} \int_T (\bar{f} + f')(\bar{g} + g') \, dt$$

$$= \frac{1}{T} \int_T (\bar{f}\bar{g} + f'\bar{g} + \bar{f}g' + f'g') \, dt$$

$$= \bar{f}\bar{g} + \bar{g} \underbrace{\frac{1}{T} \int_T f' \, dt}_{=0} + \bar{f} \underbrace{\frac{1}{T} \int_T g' \, dt}_{=0} + \overline{f'g'}$$

$$= \bar{f}\bar{g} + \underline{\underline{\overline{f'g'}}} \quad \text{usually} \neq 0, \text{ z. B. } f = g \rightarrow \overline{f'^2} \neq 0$$

$$\left. \begin{array}{l} \text{level of turbulence} \\ \text{turbulent intensity} \end{array} \right\} \text{Tu} = \frac{1}{\overline{u_\infty}} \sqrt{\frac{1}{3}(\overline{u'^2} + \overline{v'^2} + \overline{w'^2})}$$

3-D, incompressible, unsteady momentum equation

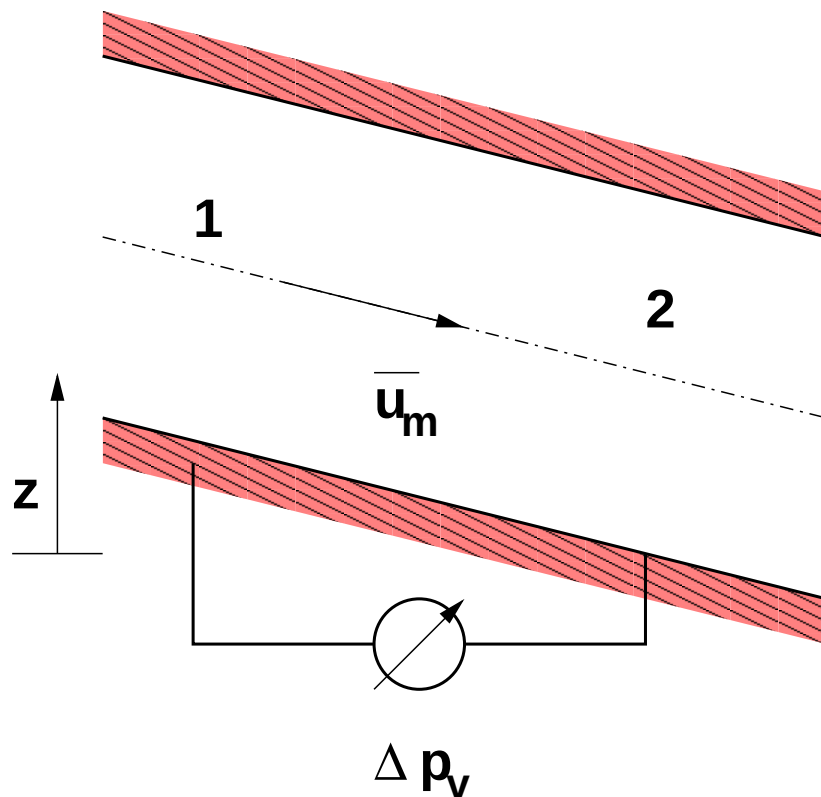
konvective term: $\frac{\partial v_k v_j}{\partial x_k}$ z. B.: $\frac{\partial uv}{\partial x}; \frac{\partial vw}{\partial y}$

for turbulent flows: average of the complete equation

$$\rightarrow \frac{\overline{\partial v_k v_j}}{\partial x_k} = \frac{\partial}{\partial x_k} = (\overline{v_k v_j} + \underbrace{\overline{v'_k v'_j}}_{\text{additional term}})$$

$-\rho \overline{v'_k v'_j}$ turbulent shear stress tensor

Bernoulli equation (Energy equation) for pipe flows with loss of the total pressure



$$p_{01} = p_{02} + \underbrace{\Delta p_v}_{\text{Total pressure loss}}$$

$$p_1 + \frac{\rho}{2} \overline{u_{m1}}^2 + \rho g z_1 = p_2 + \frac{\rho}{2} \overline{u_{m2}}^2 + \rho g z_2 + \Delta p_v$$

$$\Delta p_v = \Sigma \left(\xi_i + \lambda_i \frac{L_i}{D_i} \right) \frac{\rho}{2} \overline{u_{m_i}}^2$$

$\zeta_i \triangleq$ pressure loss coefficient for special places,
where losses occur
(inlet, unsteady enlargement of cross section, elbow, . . .)

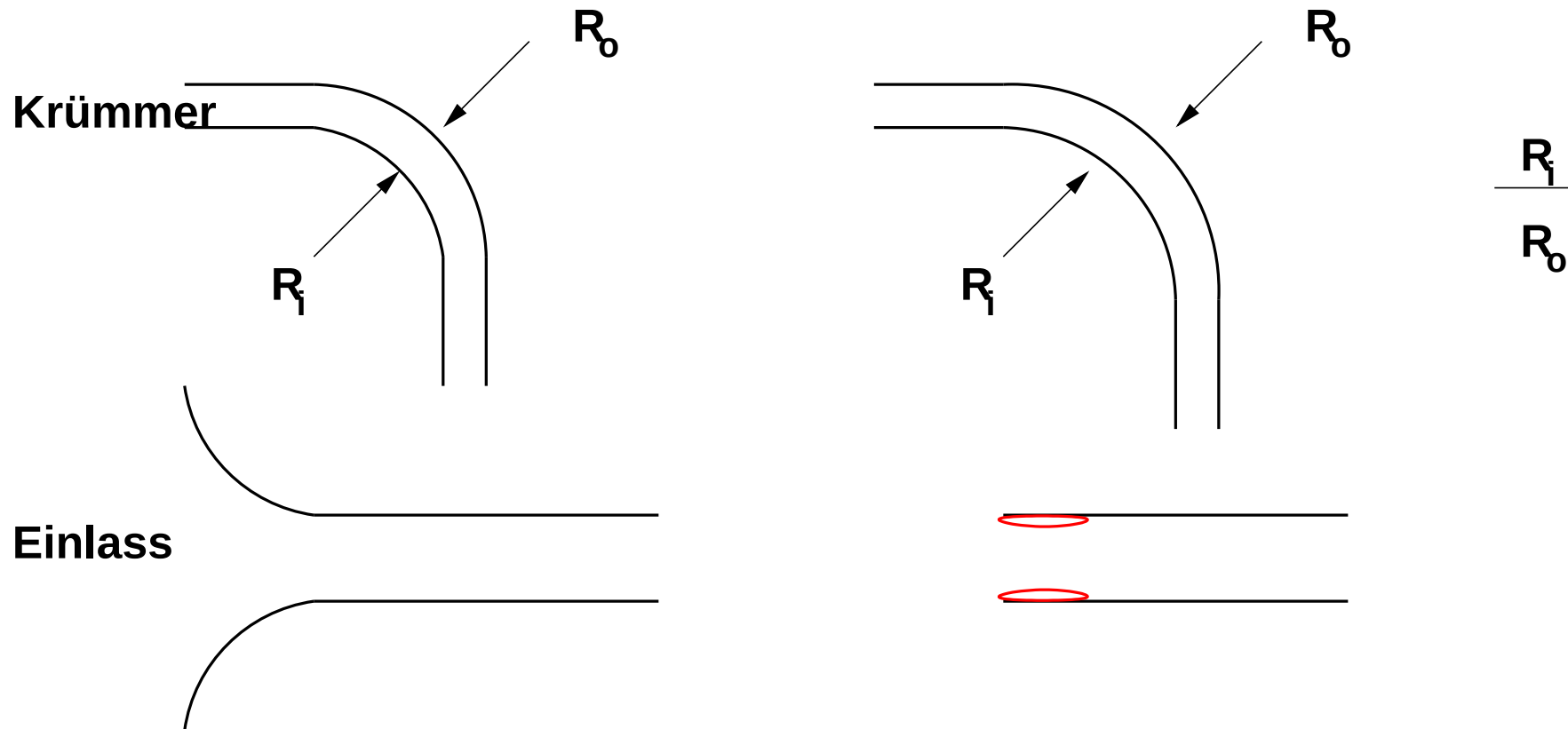
$\lambda_i \triangleq$ loss coefficient in straight pipes

$\overline{u_{m_i}} \triangleq$ average velocity

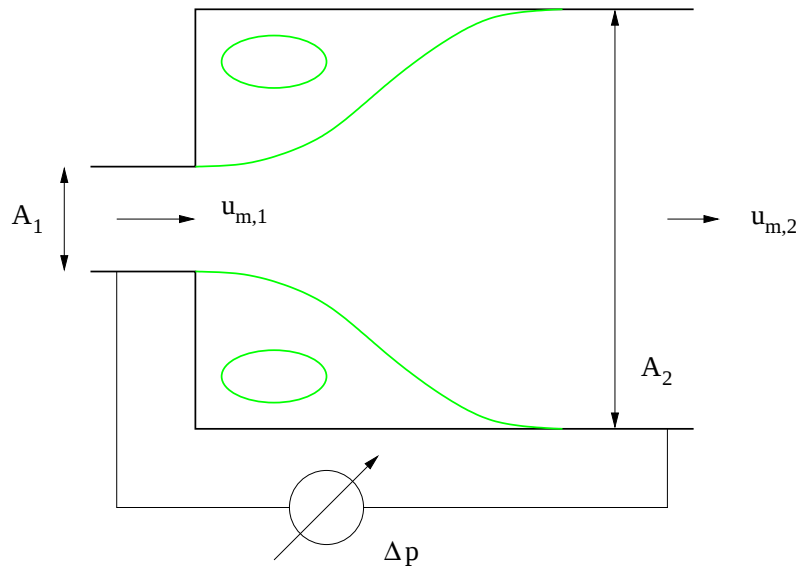
examples for pressure loss coefficients

usually: determine ζ from experiments

$$\xi = \xi(\text{Re, geometry})$$



unsteady enlargement of cross section



Carnot equation

$$\zeta_E = \frac{\Delta p}{\frac{\rho}{2} u_{m1}^2} = \left(1 - \frac{A_1}{A_2}\right)^2$$

laminar flow, inlet, circular pipes

$$\rightarrow 1.12 \leq \zeta_e \leq 1.45 \quad \text{experimental}$$

pressure loss coefficients for pipes (smooth pipes)

$$\text{Re} = \frac{\bar{u} \rho D}{\eta}$$

- laminar: ($\text{Re} \leq 2.300$) $\lambda = \frac{C}{\text{Re}}$

$C = 64$ for circular cross-sections (Hagen-Poiseuille)

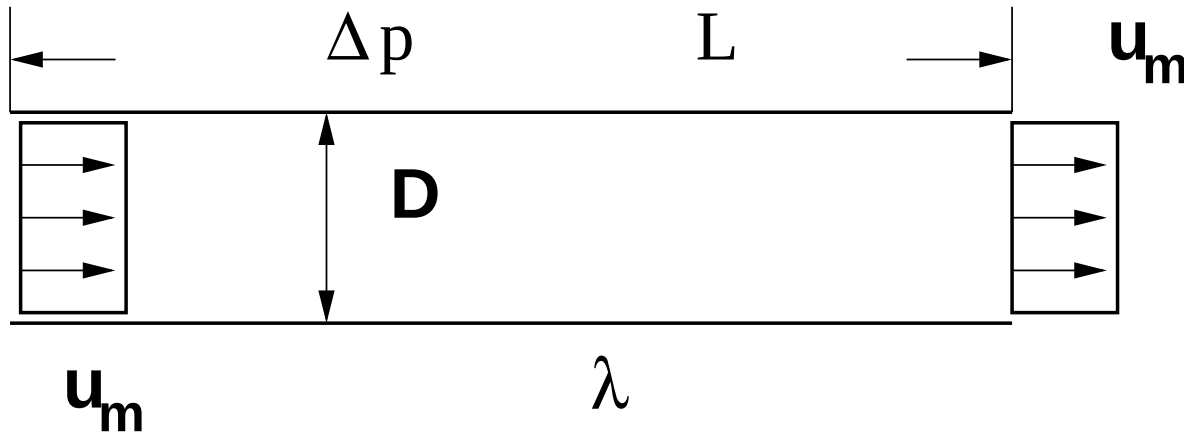
- turbulent: Blasius ($2.300 \leq \text{Re} \leq 10^5$)

$$\lambda = \frac{0.316}{\sqrt[4]{\text{Re}}}$$

iterative solution: Prandtl: $\frac{1}{\sqrt{\lambda}} = 2 \log(\text{Re} \sqrt{\lambda}) - 0.8$

Reference velocity

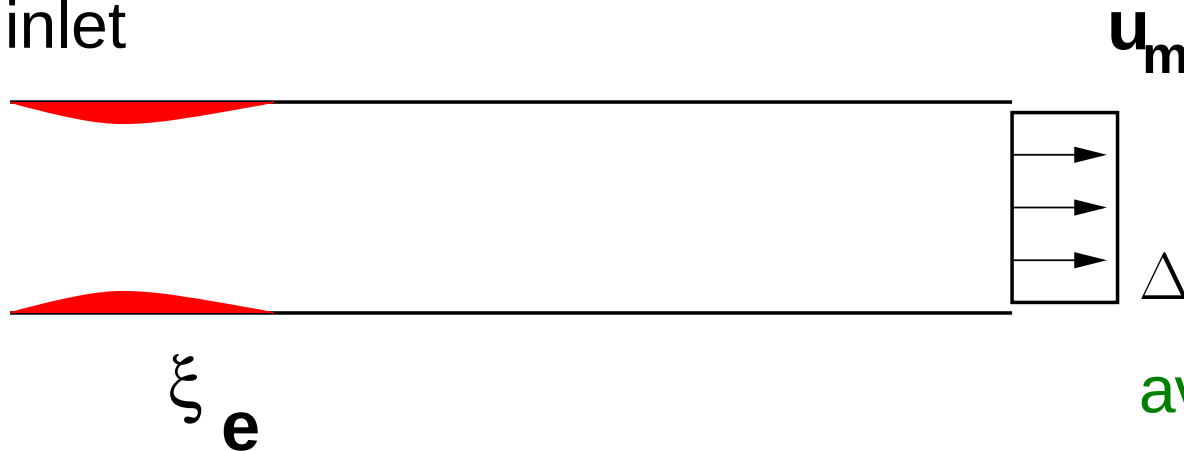
viscous effects in the pipe



$$\Delta p = \lambda \frac{L}{D} \frac{\rho}{2} \overline{u_m^2}$$

average pipe velocity

inlet

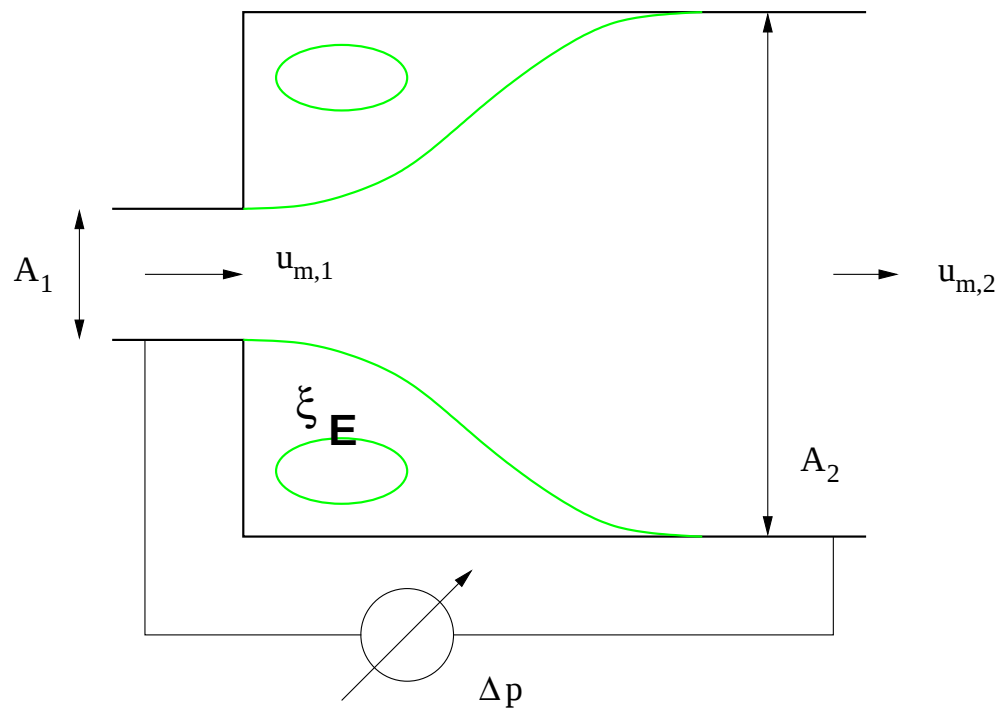


$$\Delta p_v = \xi_e \frac{\rho}{2} \overline{u_m^2}$$

average pipe velocity

Reference velocity

unsteady change of cross section

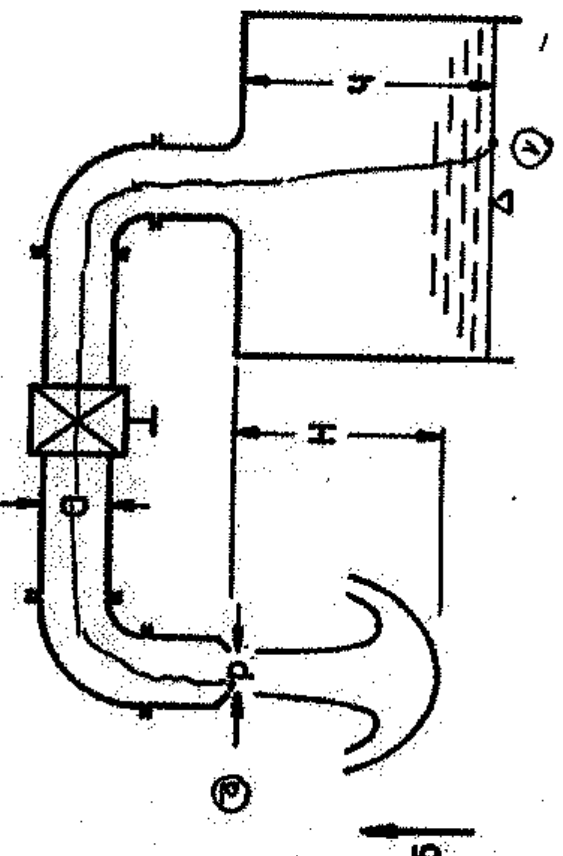


$$\Delta p_v = \xi_E \frac{\rho}{2} \overline{u_{m,1}^2}$$

incoming velocity

typical problem (losses)

6.8 Die Zuleitung eines Springbrunnens besteht aus vier geraden Rohrstücken der Gesamtlänge L , zwei Krümmern (Verlustbeiwert ζ_K) und einem Ventil (ζ_V).



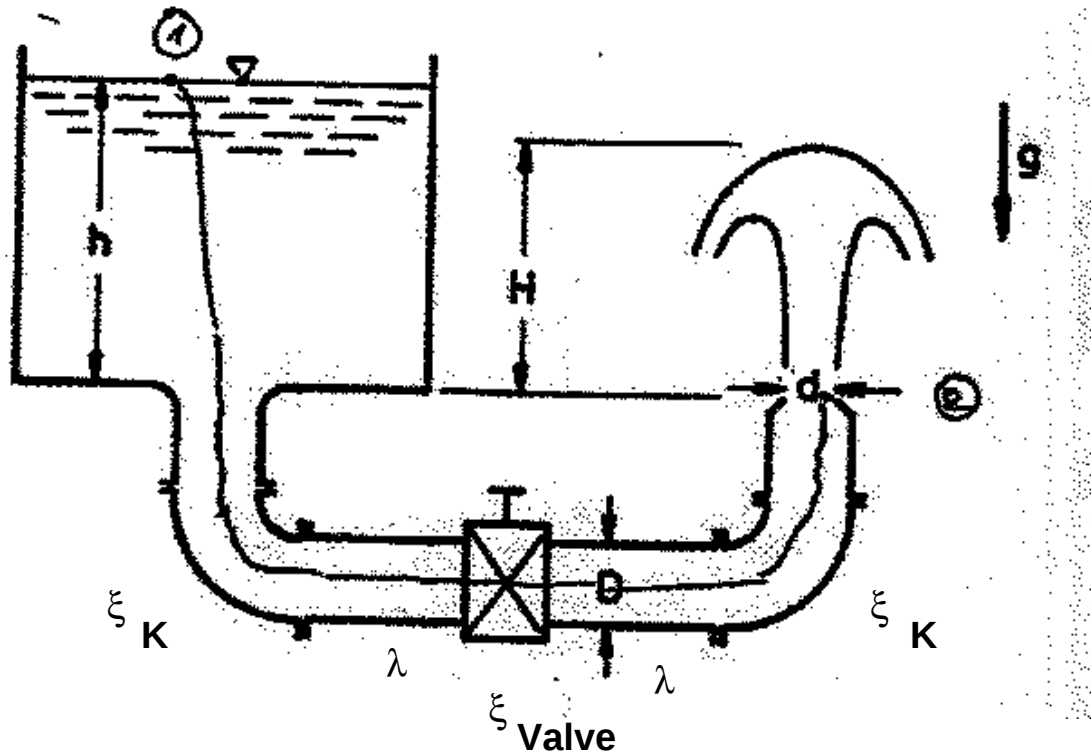
$h = 10 \text{ m}$ $D = 0,05 \text{ m}$ $L = 4 \text{ m}$ $\zeta_K = 0,25$
 $\zeta_V = 4,5$ $\lambda = 0,025$

Bestimmen Sie für verlustbehaftete und verlustfreie Strömung den Volumenstrom und die Höhe H für

- $d = D/2$,
- $d = D$!

Hinweise: Nehmen Sie an, daß die Strömung in der Düse und im Einlauf verlustfrei und in den Rohrstücken ausgebildet ist!

typical problem (losses)



Bemerkung:

- mechanical losses are known (ζ , λ)
- the flow in the inlet and in the nozzle is lossfree
- the flow in the pipes is fully developed

typical problem (losses)

→ Bernoulli

$$p_{01} = p_{02} + \Delta p_v$$

total available
energie

still existing
total energy in '2'

transformed
→ inner energy
total pressure loss

$$p_0 = p + \frac{\rho}{2}u^2 + \rho gz$$

$$\Delta p_v = \Sigma \left(\xi_i + \lambda_i \frac{L_i}{D_i} \right) \frac{\rho}{2} \overline{u_{m_i}}^2$$

typical problem (losses)

Bernoulli from 'd' $\rightarrow$ 'H' ($u_H = 0$)

$$p_a + \frac{\rho}{2}u_d^2 = p_a + \rho g H$$

$$\rightarrow H = \frac{u_d^2}{2g} \rightarrow \text{unknown } u_d ?$$

extended Bernoulli

$$p_{01} = p_a + \rho g h = p_a + \underbrace{\frac{\rho}{2}u_d^2 + \frac{\rho}{2}u_{mD}^2 (2\xi_K + \xi_v + \lambda \frac{L}{D})}_{K}$$

$\nearrow$
 $\uparrow$

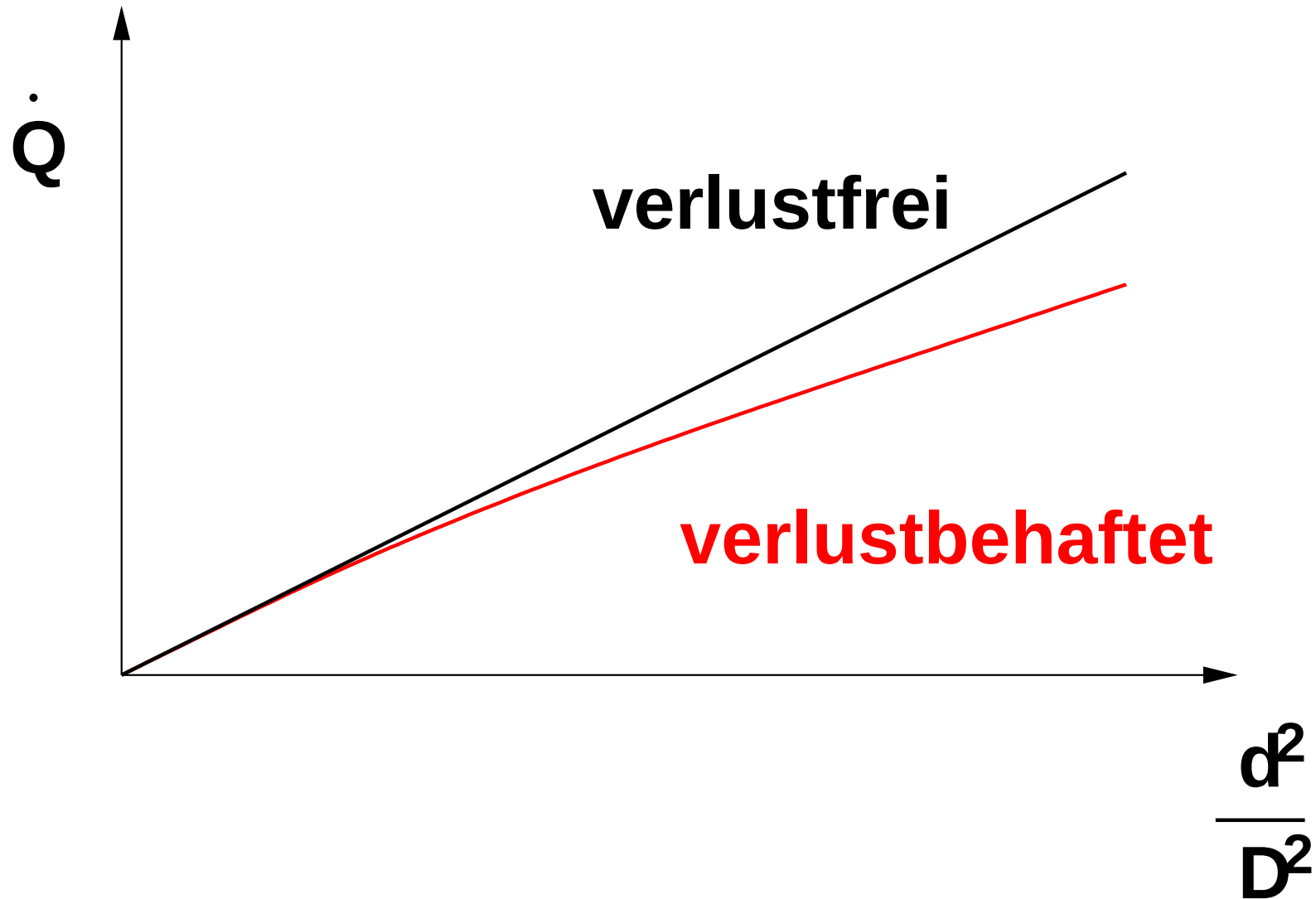
nozzle velocity
pipe velocity

continuity: $u_{mD} A_D = u_d A_d \rightarrow u_{mD} = u_d \left(\frac{d}{D} \right)^2$

typical problem (losses)

lossfree	with losses
$\xi_K = \xi_v = \lambda = 0$	$\rho gh = \frac{\rho}{2} u_d^2 + \rho 2 u_d^2 \left(\frac{d}{D} \right)^4 K$
$u_d = \sqrt{2gh}$	$u_d = \sqrt{\frac{2gh}{1 + \left(\frac{d}{D} \right)^4 K}}$
Volume flux $\dot{Q} = \frac{\pi}{4} d^2 u_d$	
$\dot{Q} = \frac{\pi}{4} \sqrt{2gh} D^2 \frac{d^2}{D^2}$	$\dot{Q} = \frac{\pi}{4} \sqrt{2gh} D^2 \frac{d^2}{D^2} \frac{\left(\frac{d}{D} \right)^2}{\sqrt{1 + \left(\frac{d}{D} \right)^4 K}}$
$\dot{Q} \sim \left(\frac{d}{D} \right)^2$	

typical problem (losses)



typical problem (losses)

ceiling of the fountain

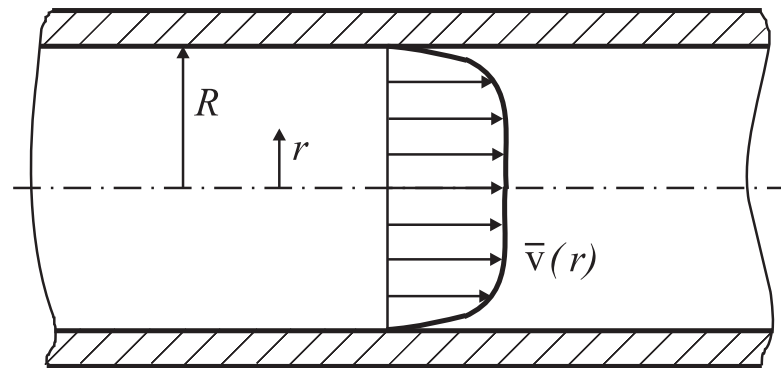
$$H = \frac{u_d^2}{2g}$$

no losses	with losses
$H = h$	$H = \frac{h}{1 + \left(\frac{d}{D}\right)^4 K}$
	influence of $\frac{d}{D}$
	$\frac{d}{D} \downarrow \rightarrow H \uparrow$

The velocity profile in a fully developed flow in a pipe with a smooth surface can be approximated with the potential law:

$$\frac{\bar{v}}{\bar{v}_{max}} = \left(1 - \frac{r}{R}\right)^{\frac{1}{n}}, \text{ mit } n = n(Re).$$

Re	n
$1 \cdot 10^5$	7
$6 \cdot 10^5$	8
$1.2 \cdot 10^6$	9
$2 \cdot 10^6$	10



- a) Use the continuity equation to compute the relation between the average velocity $\bar{v}_m$ and the maximum velocity $\bar{v}_{max}$, i. e. $\frac{\bar{v}_m}{\bar{v}_{max}} = f(n)$.
- b) At what position $\frac{r}{R}$ is $\bar{v}(r/R) = \bar{v}_m$?
- c) How can the results of a) and b) be used, if the volume flux shall be measured?

The ratio between the average and the maximum velocity is

$$\frac{\bar{v}_m}{\bar{v}_{max}} = 2 \int_0^1 \xi (1 - \xi)^{\frac{1}{n}} d\xi = \frac{2n^2}{(n+1)(2n+1)} \quad \text{mit} \quad \xi = \frac{r}{R} .$$

The integral is solved using partial integration. The average velocity is at a distance

$$\frac{r_m}{R} = 1 - \left(\frac{\bar{v}_m}{\bar{v}_{max}} \right)^n$$

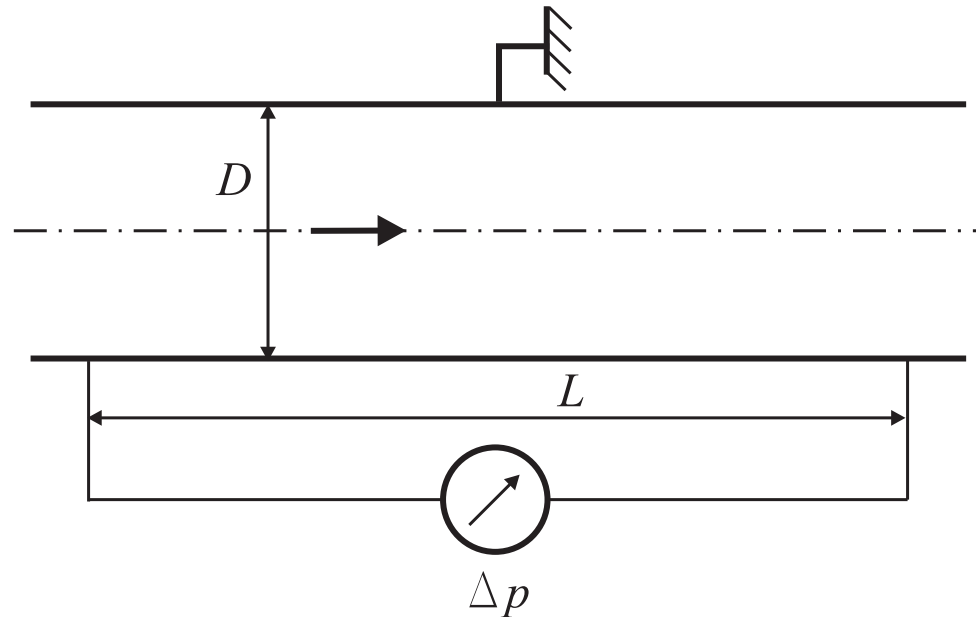
see table.

Re	n	$\bar{v}_m/\bar{v}_{max}$	r_m/R
$1 \cdot 10^5$	7	0.8166	0.7577
$6 \cdot 10^5$	8	0.8366	0.76
$1.2 \cdot 10^6$	9	0.8526	0.762
$2 \cdot 10^6$	10	0.8658	0.7633

Measuring $\bar{v}(r)$ at a distance $R - r_m$ from the wall, and with the known $\bar{v}_{max}$ the average velocity can be determined, and the volume flux $\dot{V} = \bar{v}_m \pi R^2$ can be computed.

10.6

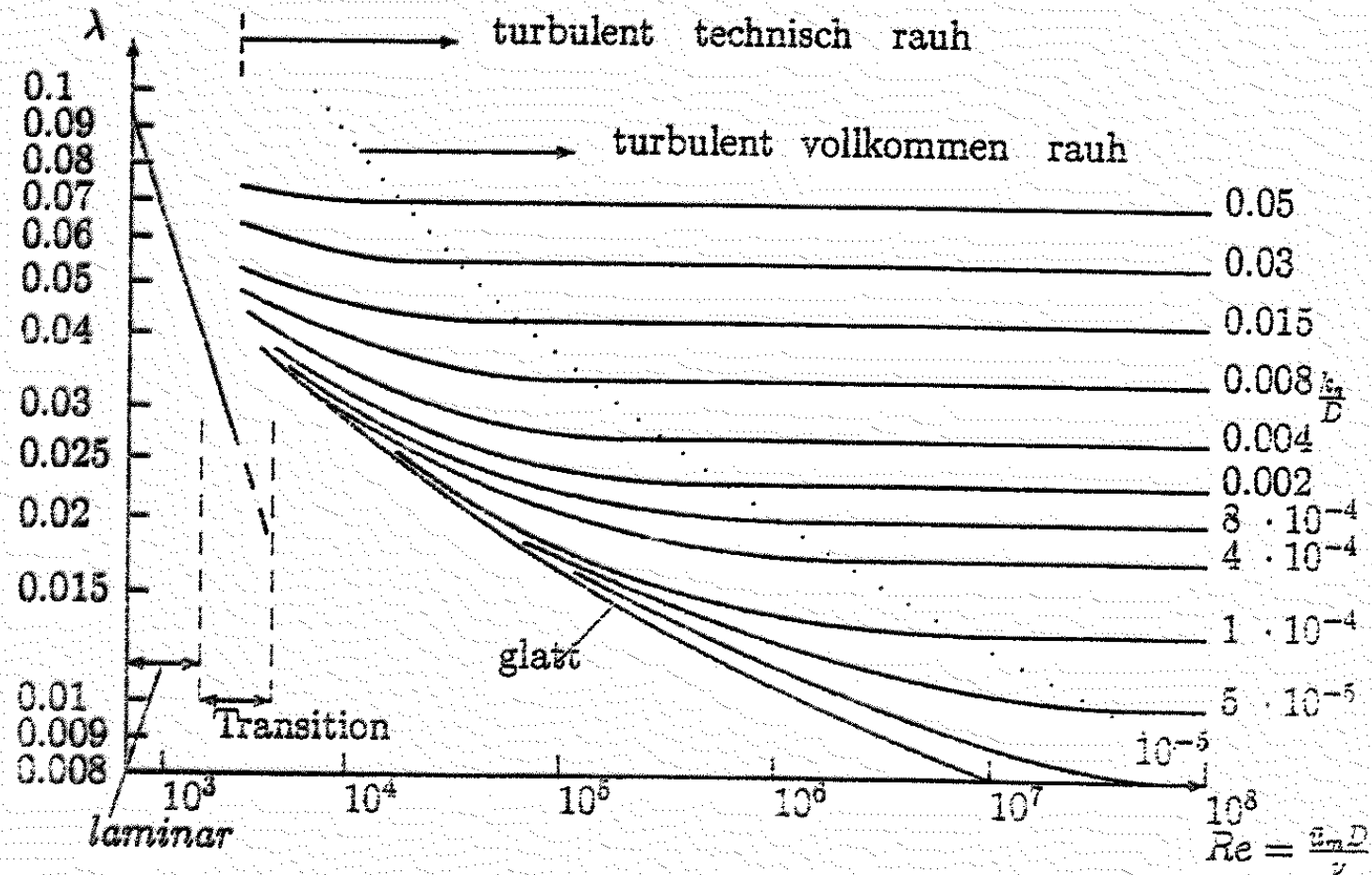
The pressure decrease Δp along L is measured in a fully developed pipe flow with the volume flux $\dot{V}$.

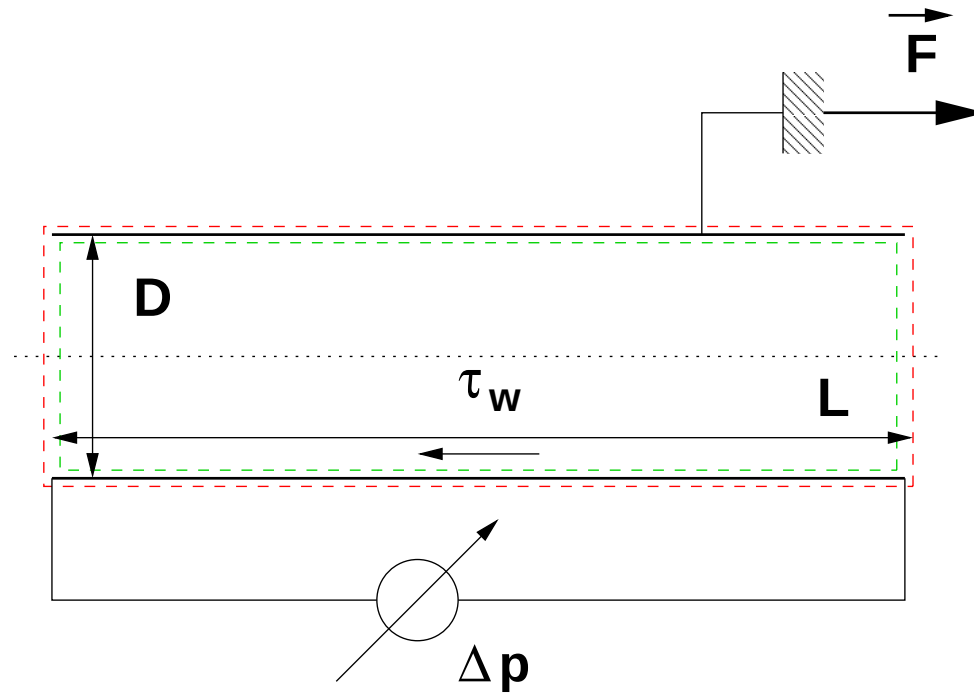


$$\begin{aligned} \dot{V} &= 0,393 \text{ m}^3/\text{s} & L &= 100 \text{ m} & D &= 0,5 \text{ m} & \Delta p &= 12820 \text{ N/m}^2 & \rho &= \\ & & & & & & & & 900 \text{ kg/m}^3 \\ \eta &= 5 \cdot 10^{-3} \text{ Ns/m}^2 \end{aligned}$$

Determine

- a) the skin-friction coefficient,
- b) the equivalent roughness of the pipe,
- c) the wall shear stress and the force of the support.
- d) What is the pressure decrease, if the pipe is smooth?





a)

$$\Delta p = \lambda \frac{L}{D} \frac{\rho}{2} \bar{u}_m^2$$

$$\dot{V} = \bar{u}_m \frac{\pi D^2}{4}$$

$$\Rightarrow \lambda = \frac{\pi^2 \Delta p D^5}{8 \rho L \dot{V}^2} = 0,0356$$

$$\text{b)} \quad Re = \frac{\rho \bar{u}_m D}{\eta} = 1,8 \cdot 10^5$$

$$\frac{k_s}{D} = 0,0083 \quad (\text{from Moody diagram})$$
$$\implies k_s = 4,2 \text{ mm}$$

c)

momentum equation for the inner control surface:

$$\Delta p \frac{\pi D^2}{4} - \tau_W \pi D L = 0$$

$$\implies \tau_W = \Delta p \frac{D}{4L} = 16 \text{ N/m}^2$$

momentum equation for the outer control surface:

$$F = -\Delta p \frac{\pi D^2}{4} = -2517 \text{ N}$$

d) $\lambda = 0,016$ (from diagram)

$$\Rightarrow \Delta p = 5,8 \cdot 10^3 \text{ N/m}^2$$